

Basic Modeling With Simple Examples

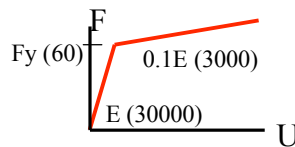
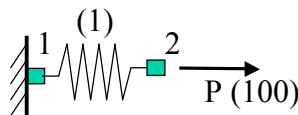
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UC Berkeley

OpenSees Days Shanghai 2011



Spring Example - Load Control

a.tcl



```

# create the model builder
model Basic -ndm 1 -ndf 1
# create 2 nodes
node 1 0.0
node 2 0.0
# fix node 1
fix 1 1
# create material
set Fy 60.0
set E 30000.0
set b 0.1
uniaxialMaterial Steel01 1 $Fy $E $b
# create element
element zeroLength 1 1 2 -mat 1 -dir 1
# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
  load 2 $P
}

# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-6 6 0
algorithm Newton
system ProfileSPD
integrator LoadControl 0.1
analysis Static

# perform the analysis
analyze 10

# Output
print node 2
set exact [expr $Fy/$E + (100-$Fy)/($b*$E)]
set res [lindex [nodeDisp 2] 0]
if {$sexact == $res} {
  puts "Exact (Sexact) EQUALS Result ($res)"
} else {
  puts "Exact (Sexact) NOT EQUAL Result ($res)"
}
  
```

```

Terminal — bash — 80x19
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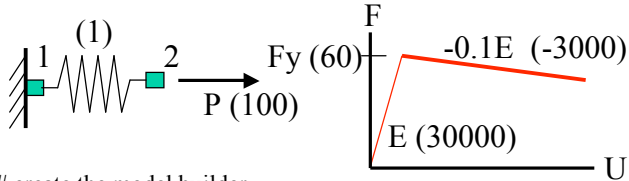
Node: 2
Coordinates : 0
Disps: 0.0153333
Velocities : 0
unbalanced Load: 100
ID : 0

Exact (0.015333333333333334) IS NOT EQUAL TO Result (0.015333333333333250)
fmk:~/Desktop/Workshops/ChinWorkshop2011/exam

```



Computers cannot represent all numbers exactly
and
Computer math involves roundoff




b.tcl

```

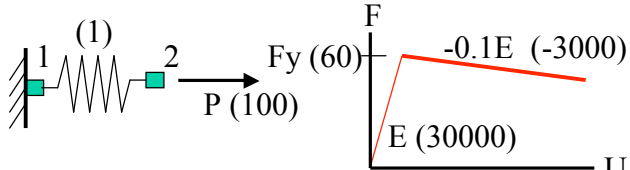
# create the model builder
model Basic -ndm 1 -ndf 1
# create 2 nodes
node 1 0.0
node 2 0.0
# fix node 1
fix 1 1
# create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 $Fy $E $b
# create element
element zeroLength 1 1 2 -mat 1 -dir 1
# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
  load 2 $P
}
# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-6 6 0
algorithm Newton
system ProfileSPD
integrator LoadControl 0.1
analysis Static
# perform the analysis
analyze 10
# Output
print node 2

```



change **system ProfileSPD** to **system BandGen**

change **LoadControl 0.1** to **LoadControl 0.0999999**



c.tcl

```

# create the model builder
model Basic -ndm 1 -ndf 1
# create 2 nodes
node 1 0.0
node 2 0.0
# fix node 1
fix 1 1
# create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 $Fy $E $b
# create element
element zeroLength 1 1 2 -mat 1 -dir 1
# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
  load 2 $P
}
# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-6 6 0
algorithm Newton
system BandGen
integrator LoadControl 0.09999
analysis Static
# perform the analysis
analyze 10
# Output
print node 2

```

```

Terminal - bash - 104x22
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WARNING: CTestNormDispIncr::test() - failed to converge
after: 6 iterations
NewtonRaphson::solveCurrentStep() -the ConvergenceTest object failed in test()
StaticAnalysis::analyze() - the Algorithm failed at iteration: 6 with domain at load factor 0.7
OpenSees > analyze failed, returned: -3 error flag

Node: 2
Coordinates : 0
Disps: 0.002
Velocities : 0
unbalanced Load: 60
ID : 0

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$

```

With a Yield Strength of 60
This is as far as we can push the
Model using LoadControl

We can go further using a Displacement Control scheme

Spring Example - Displacement Control

d.tcl

```

# create the model builder
model Basic -ndm 1 -ndf 1
# create 2 nodes
node 1 0.0
node 2 0.0
# fix node 1
fix 1 1
# create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 $Fy $E $b
# create element
element zeroLength 1 1 2 -mat 1 -dir 1
# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
  load 2 $P
}

# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-12 6 0
algorithm Newton
system BandGen
integrator DisplacementControl 2 1 0.001
analysis Static

# perform the analysis & print results
for {set i 0} {$i < 10} {incr i 1} {
  analyze 1
  set factor [getTime]
  puts "[expr $factor*$P] [lindex [nodeDisp 2] 0]"
}

print node 2

```

```

Terminal — bash — 92x30
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ OpenSees d.tcl

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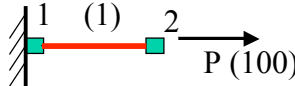
30.0 0.00100000000000000002
60.0 0.00200000000000000004
56.999999999999999 0.00300000000000000006
54.0 0.00400000000000000008
51.0 0.00500000000000000010
48.0 0.00600000000000000012
45.0 0.00700000000000000015
42.0 0.00800000000000000017
39.0 0.009000000000000000105
36.0 0.010000000000000000194

Node: 2
Coordinates : 0
Disps: 0.01
Velocities : 0
unbalanced Load: 36
ID : 0

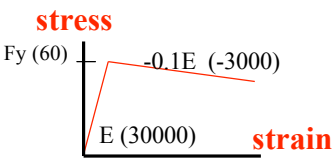
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ █

```

Truss Example - Displacement Control e.tcl



1 (1) 2
P (100)



stress
Fy (60) -0.1E (-3000)
E (30000) strain

```

# create the model builder
model Basic -ndm 1 -ndf 1

# create 2 nodes
node 1 0.0
node 2 1.0
# fix node 1
fix 1 1

# create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 $Fy $E $b

# create element
set A 1.0
element Truss 1 1 2 $A 1

# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
  load 2 $P
}

# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-12 6 0
algorithm Newton
system BandGen
integrator DisplacementControl 2 1 0.001
analysis Static

# perform the analysis & print results
for {set i 0} {$i < 10} {incr i 1} {
  analyze 1
  set factor [getTime]
  puts "[expr $factor*$P] [lindex [nodeDisp 2] 0]"
}

print node 2

```

```

Terminal — bash — 92x30
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ OpenSees e.tcl

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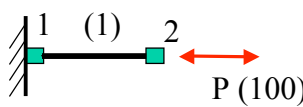
30.0 0.00100000000000000002
60.0 0.00200000000000000004
56.999999999999999 0.00300000000000000006
54.0 0.00400000000000000008
51.0 0.00500000000000000010
48.0 0.00600000000000000012
45.0 0.00700000000000000015
42.0 0.00800000000000000017
39.0 0.009000000000000000105
36.0 0.010000000000000000194

Node: 2
Coordinates : 1
Disps: 0.01
Velocities : 0
unbalanced Load: 36
ID : 0

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ █

```

Truss Example - Push & Pull



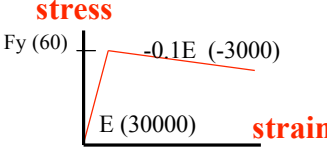
create the model builder
model Basic -ndm 1 -ndf 1

create 2 nodes
node 1 0.0
node 2 1.0
fix node 1
fix 1 1

create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 \$Fy \$E \$b

create element
set A 1.0
element Truss 1 1 2 \$A 1

create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
 load 2 \$P
}



create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-12 6 0
algorithm Newton
system BandGen
integrator DisplacementControl 2 1 0.001
analysis Static

perform the analysis & print results

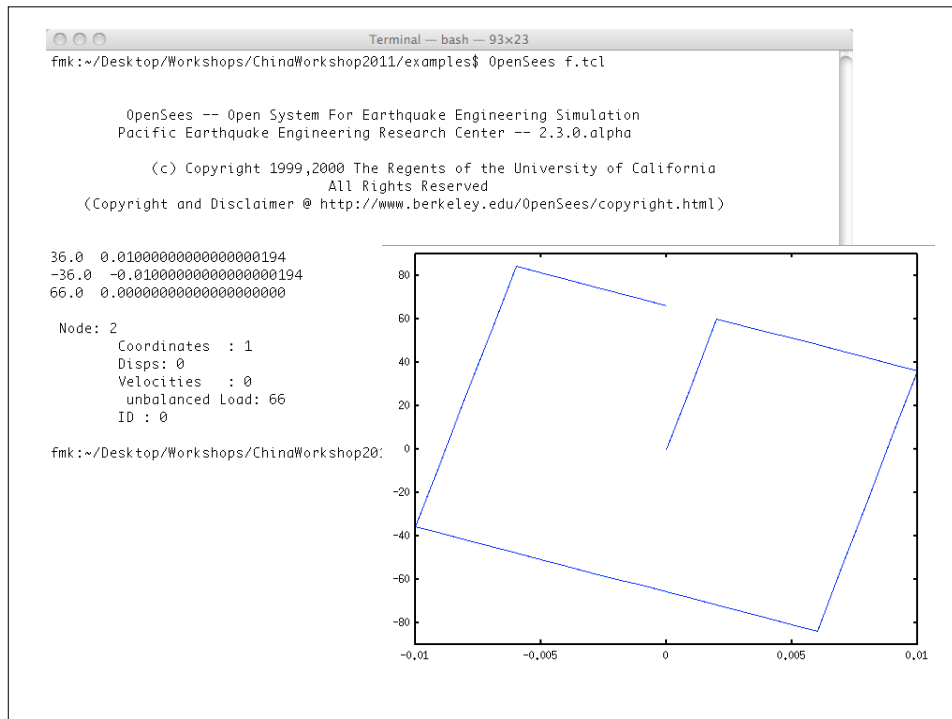
```

foreach {SnumIter SdU} {10 0.001 20 -0.001 10 0.001} {
  integrator DisplacementControl 2 1 0.001
  analyze $numIter
  set factor [getTime]
  puts "[expr $factor*$P] [index [nodeDisp 2] 0]"
}

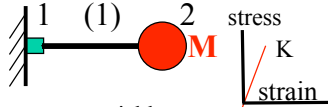
```

print node 2

f.tcl



Truss Example - Uniform Excitation j.tcl



```

# create the uniform excitation pattern
set record el_centro
source ReadRecord.tcl
ReadRecord $record.AT2 $record.dat dT nPts
timeSeries Path 1 -filePath $record.dat -dt $dT -factor $
pattern UniformExcitation 1 1 -accel 1

# create the analysis
constraints Plain
integrator Newmark 0.5 [expr 1.0/6.0]
system ProfileSPD
test NormUnbalance 1.0e-12 6 0
algorithm Newton
numberer RCM
analysis Transient

# perform analysis
set t 0.0; set ok 0.0; set maxD 0.0;
set maxT [expr (1+$nPts)*$dT];
while {$ok == 0 && $t < $maxT} {
  set ok [analyze 1 $dT]
  set t [getTime]
  set d [nodeDisp 2 1]
  if {$d > $maxD} {
    set maxD $d
  } elseif {$d < [expr -$maxD]} {
    set maxD [expr -$d]
  }
}
puts "record: $record period: $Tn damping ratio: $dampR

```

```

# set some variables
set Tn 1.0
set K 4.0
set dampR 0.02

#set some constants
set g 386.4
set PI [expr 2.0 * asin(1.0)]

#derived quantities
set Wn [expr 2.0 * $PI / $Tn]
set M [expr $K / ($Wn * $Wn)]
set c [expr 2.0*$M*$Wn*$dampR]

# create the model
model basic -ndm 1 -ndf 1
node 1 0.0
node 2 1.0 -mass $M
fix 1 1
uniaxialMaterial Elastic 1 $K 0.0
uniaxialMaterial Elastic 2 0.0 $c
uniaxialMaterial Parallel 3 1 2

element truss 1 1 2 1.0 3
set dT 0.0;
set nPts 0;

```

```

Terminal -- bash -- 81x13
examples> OpenSees j.tcl

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record: el_centro period: 1.0 damping ratio: 0.02 max disp: 5.962305018001343
examples>

```

Elastic Portal Frame - Pushover

g.tcl

```

# create the model builder
model Basic -ndm 2 -ndf 3

```

```

# create 2 nodes
node 1 0.0 0.0
node 2 360.0 0.0
node 3 0.0 144.0
node 4 360.0 144.0

```

```

# fix node 1
fix 1 1 1 1
fix 2 1 1 1

```

```

geomTransf Linear 1
geomTransf Linear 2

```

```

# create elements
source SteelWSections.tcl
set in 1.0
set E 30000

```

```

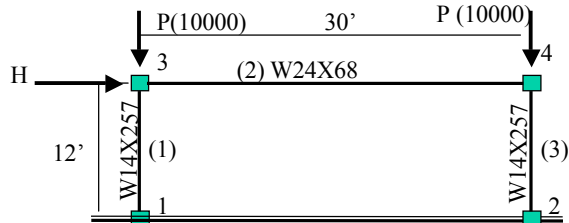
ElasticBeamWSection2d 1 1 3 W14X257 SE 1
ElasticBeamWSection2d 2 3 4 W24X68 SE 2
ElasticBeamWSection2d 3 2 4 W14X257 SE 1

```

```

# create time series and load pattern
set P 10000.0
timeSeries Constant 1
pattern Plain 1 1 {
  load 3 0.0 -$P 0.0
  load 4 0.0 -$P 0.0
}

```



```

# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-12 6 0
algorithm Newton
system ProfileSPD
integrator LoadControl 1.0
analysis Static

```

```

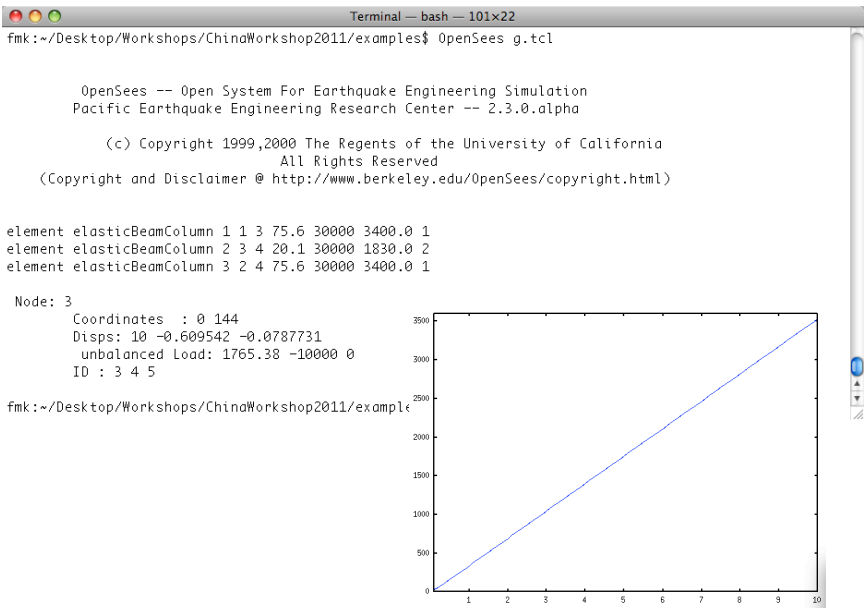
# perform the analysis
analyze 1
timeSeries Linear 2
pattern Plain 2 2 {
  load 3 1.0 0.0 0.0
  load 4 1.0 0.0 0.0
}
integrator DisplacementControl 3 1 0.1
analyze 100; print node 3

```


SteelWSections.tcl

```
proc ElasticBeamWSection2d {eleTag iNode jNode sectType E transfTag {Orient XX}} {
    global WSection
    global in
    set found 0
    foreach {section prop} [array get WSection $sectType] {
        set propList [split $prop]
        set A [expr [lindex $propList 0]*$in*$in]
        set Ixx [expr [lindex $propList 5]*$in*$in*$in*$in]
        set Iyy [expr [lindex $propList 6]*$in*$in*$in*$in]
        if {$Orient == "YY"} {
            puts "element elasticBeamColumn $eleTag $iNode $jNode $A $E $Iyy $transfTag"
            element elasticBeamColumn $eleTag $iNode $jNode $A $E $Iyy $transfTag
        } else {
            puts "element elasticBeamColumn $eleTag $iNode $jNode $A $E $Ixx $transfTag"
            element elasticBeamColumn $eleTag $iNode $jNode $A $E $Ixx $transfTag
        }
    }
}

#Winxlb/f "Area(in2) d(in) bf(in) tw(in) tf(in) Ixx(in4) Iyy(in4)"
array set WSection {
    W44X335 "98.5 44.0 15.9 1.03 1.77 31100 1200 74.7"
    W44X290 "85.4 43.6 15.8 0.865 1.58 27000 1040 50.9"
    W44X262 "76.9 43.3 15.8 0.785 1.42 24100 923 37.3"
    W44X230 "67.7 42.9 15.8 0.710 1.22 20800 796 24.9"
    W40X593 "174 43.0 16.7 1.79 3.23 50400 2520 445"
    W40X503 "148 42.1 16.4 1.54 2.76 41600 2040 277"
}
```



Terminal — bash — 101x22

```
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ OpenSees g.tcl

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element elasticBeamColumn 1 1 3 75.6 30000 3400.0 1
element elasticBeamColumn 2 3 4 20.1 30000 1830.0 2
element elasticBeamColumn 3 2 4 75.6 30000 3400.0 1

Node: 3
Coordinates : 0 144
Disps: 10 -0.609542 -0.0787731
unbalanced Load: 1765.38 -10000 0
ID : 3 4 5

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examp1t
```

The plot shows a linear relationship between two variables. The x-axis ranges from 0 to 10, and the y-axis ranges from 0 to 3500. A blue line starts at the origin (0,0) and extends to the point (10, 3500).

Elastic Portal Frame - Pushover

h.tcl

```
# create the model builder
model Basic -ndm 2 -ndf 3
```

```
# create 2 nodes
node 1 0.0 0.0
node 2 360.0 0.0
node 3 0.0 144.0
node 4 360.0 144.0
```

```
# fix node 1
fix 1 1 1 1
fix 2 1 1 1
```

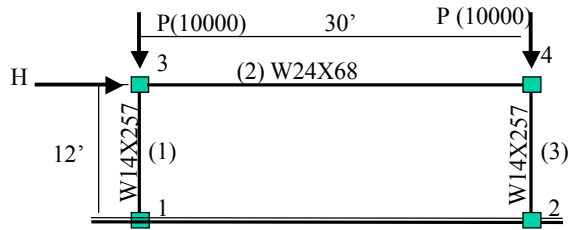
```
geomTransf PDelta 1
geomTransf Linear 2
```

```
# create elements
source SteelWSections.tcl
set in 1.0
set E 30000
```

```
ElasticBeamWSection2d 1 1 3 W14X257 $E 1
ElasticBeamWSection2d 2 3 4 W24X68 $E 2
ElasticBeamWSection2d 3 2 4 W14X257 $E 1
```

```
# create time series and load pattern
```

```
set P 10000.0
timeSeries Constant 1
pattern Plain 1 1 {
  load 3 0.0 -$P 0.0
  load 4 0.0 -$P 0.0
}
```



```
# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-12 6 0
algorithm Newton
system ProfileSPD
integrator LoadControl 1.0
analysis Static
```

```
# perform the analysis
analyze 1
```

```
timeSeries Linear 2
pattern Plain 2 2 {
  load 3 1.0 0.0 0.0
  load 4 1.0 0.0 0.0
}
```

```
integrator DisplacementControl 3 1 0.1
analyze 100; print node 3
```

```
Terminal - bash - 101x22
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ OpenSees h.tcl

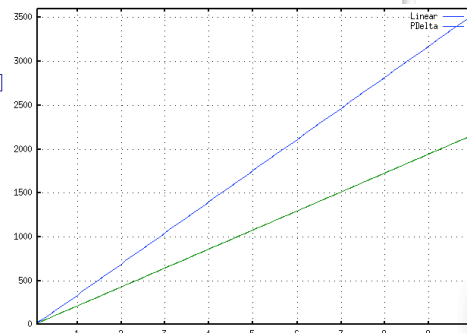
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```

```
element elasticBeamColumn 1 1 3 75.6 30000 3400.0 1
element elasticBeamColumn 2 3 4 20.1 30000 1830.0 2
element elasticBeamColumn 3 2 4 75.6 30000 3400.0 1
```

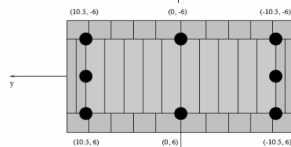
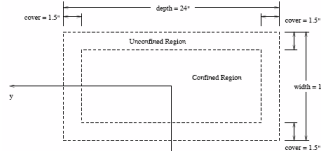
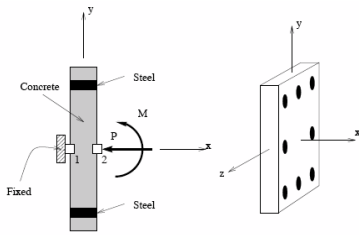
```
Node: 3
Coordinates : 0 144
Disps: 10 -0.609521 -0.0787607
unbalanced Load: 1080.3 -10000 0
ID : 3 4 5
```

```
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$
```



Moment Curvature Example

m.tcl



```

model Basic -ndm 2 -ndf 3

# Define materials 1) confined, 2) unconfined, 3 steel
uniaxialMaterial Concrete01 1 -6.0 -0.004 -5.0 -0.014
uniaxialMaterial Concrete01 2 -5.0 -0.002 0.0 -0.006
set fy 60.0; # Yield stress
set E 30000.0; # Young's modulus
uniaxialMaterial Steel01 3 $fy $E 0.01

# set some paramaters
set width 15
set depth 24
set cover 1.5
set As 0.60; # area of no. 7 bars
source RCsection2D.tcl
RCsection2D 1 $depth $width $cover 1 2 3 3 1 $As \
10 10 2

# Estimate yield curvature
# (Assuming no axial load and only top and bottom steel)
set d [expr $depth-$cover] ;# d -- from cover to rebar
set epsy [expr $fy/$E] ;# steel yield strain
set Ky [expr $epsy/(0.7*$d)]

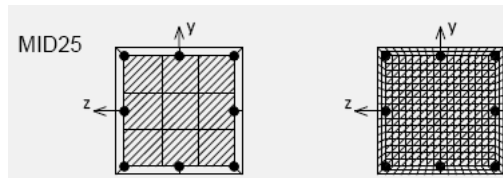
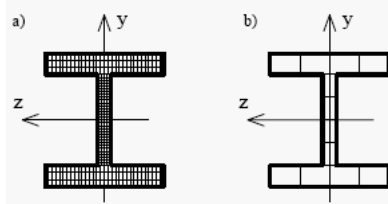
# Print estimate to standard output
puts "Estimated yield curvature: $Ky"

set P -180; # axial load
set mu 15; # Target ductility for analysis
set numIncr 100; # Number of analysis increments
# Call the section analysis procedure
source MomentCurvature.tcl
MomentCurvature 1 $P [expr $Ky*$mu] $numIncr
    
```

HOW TO MODEL FIBER SECTIONS

```

section Fiber $secTag {
  fiber <fiber arguments>
  patch <patch arguments>
  layer <layer arguments>
}
    
```



RCsection2D.tcl

RCsection2D.tcl

```
proc RCsection2D {id h b cover coreID coverID steelID numBarsTB numBarsS barArea nfCore nfCoverS
nfCoverTB} {
```

```
# some variables derived from the parameters
```

```
set y1 [expr $h/2.0]
```

```
set z1 [expr $b/2.0]
```

```
set As $barArea
```

```
section Fiber 1 {
```

```
# Create the concrete core fibers
```

```
patch rect $coreID $nfCore 1 [expr $cover-$y1] [expr $cover-$z1] [expr $y1-$cover] [expr $z1-$cover]
```

```
# Create the concrete cover fibers (top, bottom, left, right)
```

```
patch rect $coverID $nfCoverS 1 [expr -$y1] [expr $z1-$cover] $y1 $z1
```

```
patch rect $coverID $nfCoverS 1 [expr -$y1] [expr -$z1] $y1 [expr $cover-$z1]
```

```
patch rect $coverID $nfCoverTB 1 [expr -$y1] [expr $cover-$z1] [expr $cover-$y1] [expr $z1-$cover]
```

```
patch rect $coverID $nfCoverTB 1 [expr $y1-$cover] [expr $cover-$z1] $y1 [expr $z1-$cover]
```

```
# Create the reinforcing fibers (left, middle, right)
```

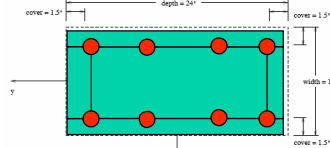
```
layer straight $steelID $numBarsTB $As [expr $y1-$cover] [expr $z1-$cover] [expr $y1-$cover] [expr $cover-$z1]
```

```
layer straight $steelID $numBarsTB $As [expr $cover-$y1] [expr $z1-$cover] [expr $cover-$y1] [expr $cover-$z1]
```

```
layer straight $steelID $numBarsS $As [expr $y1-$cover] [expr $z1-$cover] [expr $cover-$y1] [expr $cover-$z1]
```

```
layer straight $steelID $numBarsS $As [expr $y1-$cover] [expr $cover-$z1] [expr $cover-$y1] [expr $z1-$cover]
```

```
}
```



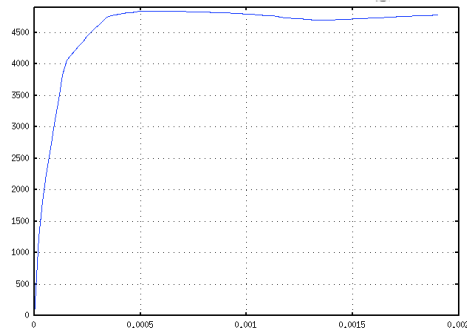
```
examples> OpenSees m.tcl
```

```
OpenSees -- Open System For Earthquake Engineering Simulation
Pacific Earthquake Engineering Research Center -- 2.3.0.alpha
```

```
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(Copyright and Disclaimer @ http://www.berkeley.edu/OpenSees/copyright.html)
```

```
Estimated yield curvature: 0.000126984126984127
```

```
examples> 
```



Any Questions?